

General Discussion on Consolidation Theory and Testing

REFERENCE: Leroueil, S. and Kabbaj, M., "General Discussion on Consolidation Theory and Testing," *Consolidation of Soils: Testing and Evaluation*, ASTM STP 892, R. N. Yong and F. C. Townsend, Eds., American Society for Testing and Materials, Philadelphia, 1986, pp. 719–723.

In the past 15 years, continuous oedometer testing techniques have been proposed to determine the compressibility characteristics of clays. These new tests are usually faster than the conventional oedometer test and, moreover, give continuous stress–strain curves. It is thus logical to wonder in 1985 whether or not we should abandon the conventional test in favor of continuous tests.

Essentially for the above reasons the authors would answer yes. Owing to strain rate effects on the behavior of natural clays, however, the results obtained from a "new" test will generally be different from those obtained from the conventional test (Fig. 1). Since our past experience is based entirely on the conventional test, it is necessary to calibrate the "new" test before using it in practice.

Various studies show that the effect of strain rate in natural clays is a very general phenomenon. As indicated in Table 1, strain rate effects were evidenced on a variety of clays with plasticity indices (I_p) between 8 and 105, liquidity indices (I_L) between 0.5 and 2.7, and conventional preconsolidation pressures ($\sigma'_{p\text{conv}}$) between 47 and 940 kPa. The strain rate effect is particularly evident in the normally consolidated range and at the preconsolidation pressure.

Considering Champlain clays, Leroueil et al [1] observed that the preconsolidation pressure–strain rate (σ'_p – $\dot{\epsilon}_v$) relationships had similar shapes. The σ'_p – $\dot{\epsilon}_v$ curves were thus normalized with respect to a reference strain rate taken arbitrarily equal to $4 \times 10^{-6} \text{ s}^{-1}$ (Fig. 2). As a first approximation, the resulting general relationship can be written:

$$\frac{\sigma'_p}{\sigma'_p(\dot{\epsilon}_v = 4 \times 10^{-6} \text{ s}^{-1})} = f(\dot{\epsilon}_v)$$

¹ Department of Civil Engineering, Laval University, Pavillon Pouliot, Quebec, Canada.

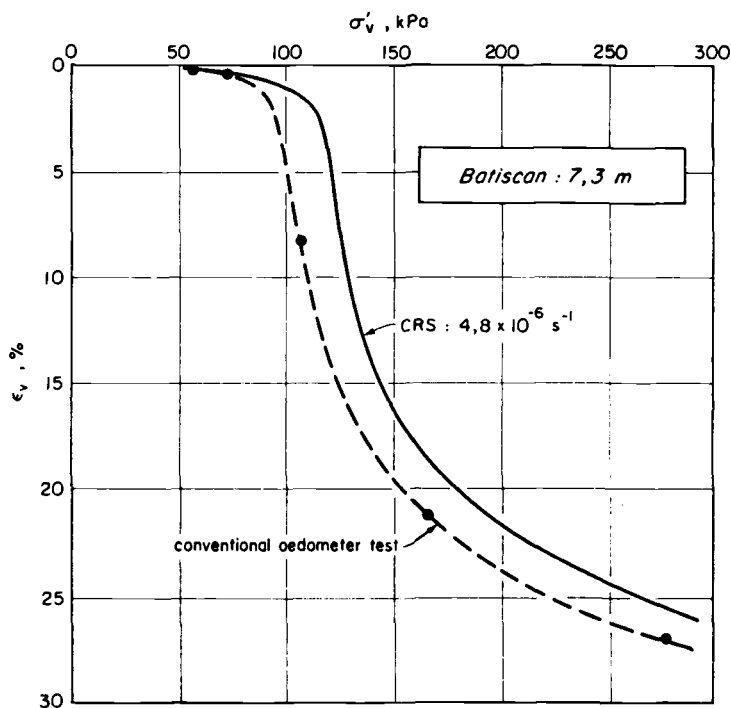


FIG. 1—Typical compressibility curves obtained from conventional and CRS tests.

TABLE 1—Characteristics of clays presenting strain rate effects.

| Clays | I_p | I_L | $\sigma'_{p \text{ conv}}$ (kPa) | S_r | Reference |
|------------------------------|----------|------------|-------------------------------------|-----------|----------------|
| <i>Champlain clays:</i> | | | | | |
| Ottawa clay | 8 | 4 | 480 | >50 | 6 |
| 14 clays | 19 to 43 | 0.9 to 2.7 | 47 to 270 | 15 to 108 | 1 |
| Louisville | 33 | 1.7 | 88 | 20 | 4 |
| <i>Other Canadian clays:</i> | | | | | |
| Saint Jean-Vianney | 16 | 1.4 | 940 | ≈100 | 7 |
| Winnipeg | 35 to 55 | ≈0.5 | ? | 3 | 8 |
| Broadback | 11 | 2.1 | 175 | ≈200 | authors' files |
| <i>Other clays:</i> | | | | | |
| Bäckebo (Sweden) | 65 | 1.04 | ≈70 | 25 | 9 |
| Belfast (N. Ireland) | 20 to 40 | ≈0.75 | ? | 8 | 8 |
| Vallda (Sweden) | ≈105 | ≈0.76 | ? | ? | 3 |

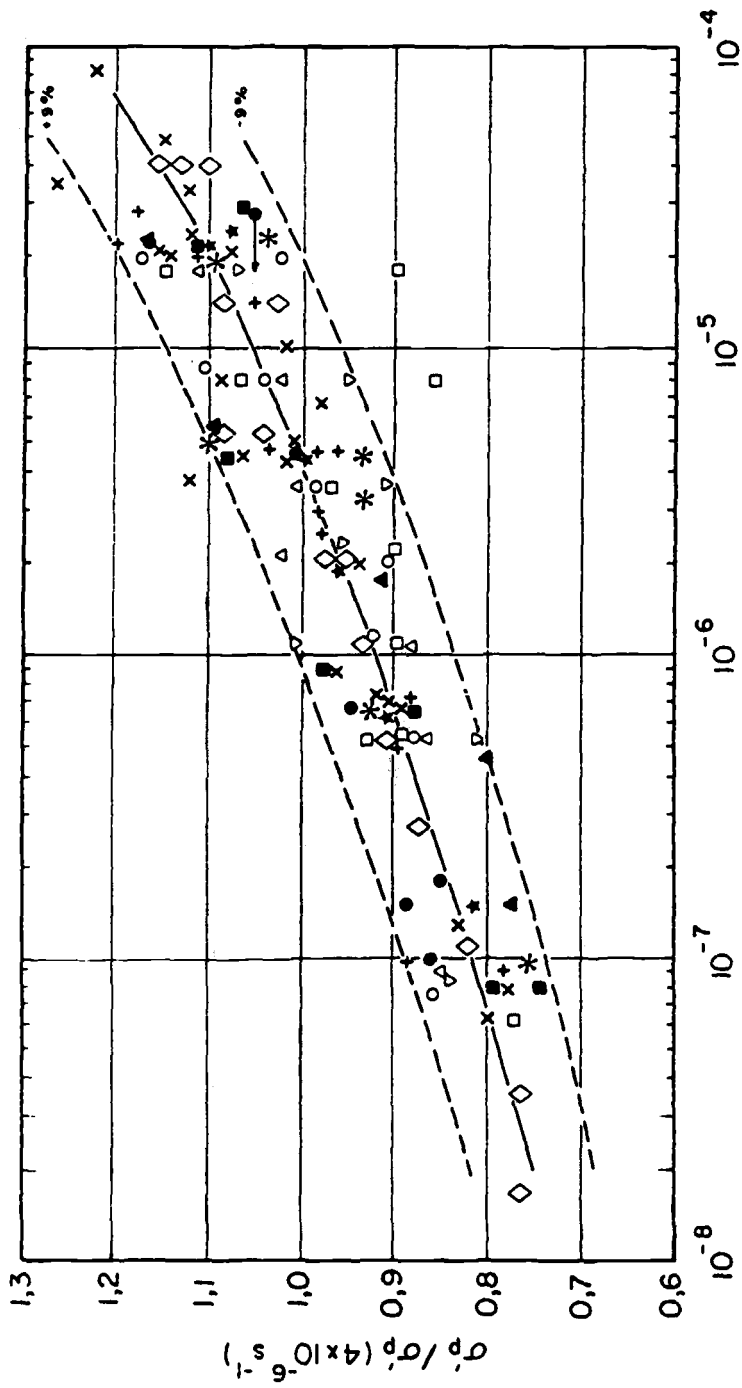


FIG. 2.—Normalized preconsolidation pressure—strain rate relationship [1].

For conventional tests, it appears that the preconsolidation pressure of Champlain clays corresponds typically to a strain rate of 10^{-7} s^{-1} . Similar observations were made by Larsson [2] and Larsson and Sällfors (Fig. 8, [3]) on Swedish clays and by Silvestri et al (Table 2, [4]) on a Champlain clay.

From these remarks, it follows that the preconsolidation pressure obtained at a given strain rate can be related to the conventional preconsolidation pressure by the relation

$$\sigma'_{p \text{ conv}} = \frac{\sigma'_p(\dot{\epsilon}_v)}{\alpha_2}$$

Figure 3 shows the α_2 - $\dot{\epsilon}_v$ relationship obtained for Champlain clays. For example, for a strain rate of $2 \times 10^{-6} \text{ s}^{-1}$, as used in Sweden, the α_2 factor would be equal to 1.18.

Moreover, Leroueil et al [5] found that the effective stress-strain-strain rate relationship is unique. Therefore, not only the preconsolidation pressure, but the whole stress-strain curve, must be corrected by a strain rate factor α_2 .

In conclusion, we believe that we are now ready to use continuous tests in Southern Quebec. Owing to its simplicity and to the important fact that the α_2 coefficient is the same during the entire test, we tend to favor the CRS test.

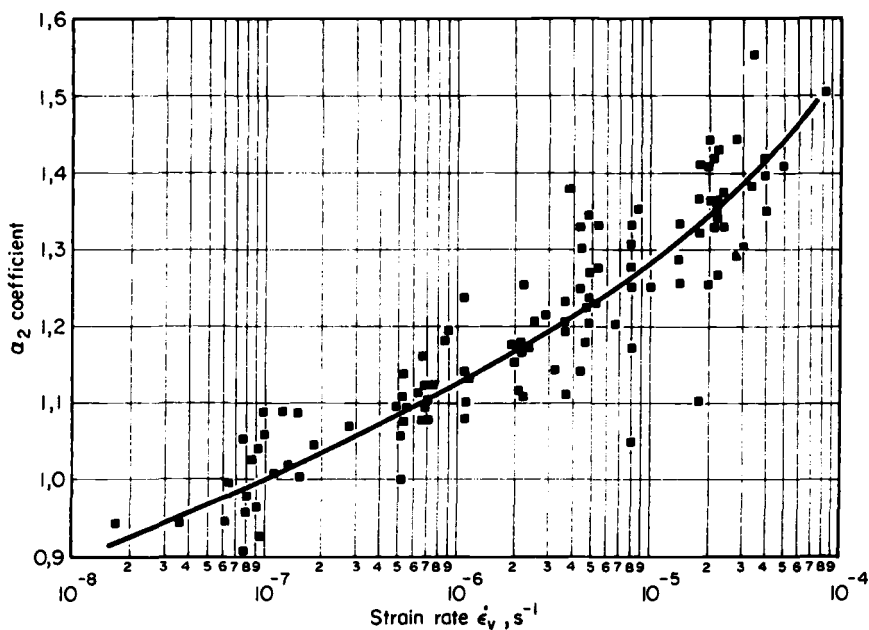


FIG. 3—Variation of the α_2 coefficient with strain rate [1].

References

- [1] Leroueil, S., Tavenas, F., Samson, L., and Morin, P., "Preconsolidation Pressure of Champlain clays: Part II—Laboratory Determination," *Canadian Geotechnical Journal*, Vol. 20, No. 4, 1983, pp. 803–816.
- [2] Larsson, R., "Drained Behaviour of Swedish Clays," Report 12, Swedish Geotechnical Institute, 1981.
- [3] Larsson, R. and Sällfors, G., "Automatic Continuous Consolidation Testing in Sweden," this publication, pp. 299–328.
- [4] Silvestri, V., Yong, R. N., Soulié, M., and Gabriel, F., "Controlled-Strain, Controlled-Gradient, and Standard Consolidation Testing of Sensitive Clays," this publication, pp. 433–450.
- [5] Leroueil, S., Kabbaj, M., Tavenas, F., and Bouchard, R., "Stress–Strain–Strain Rate Relation for the Compressibility of Sensitive Natural Clays," *Geotechnique*, Vol. 35, No. 2, 1985, pp. 159–180.
- [6] Crawford, C. B., "The Resistance of Soil Structure to Consolidation," *Canadian Geotechnical Journal*, Vol. 2, No. 2, 1965, pp. 90–97.
- [7] Vaid, Y. P., Robertson, P. K., and Campanella, R. G., "Strain Rate Behaviour of Saint-Jean-Vianney Clay," *Canadian Geotechnical Journal*, Vol. 16, No. 1, 1979, pp. 34–42.
- [8] Graham, J., Crooks, J. H. A., and Bell, A. L., "Time Effects on the Stress–Strain Behaviour of Natural Soft Clays," *Geotechnique*, Vol. 33, No. 3, 1983, pp. 327–340.
- [9] Sällfors, G., "Preconsolidation Pressure of Soft High Plastic Clays," Ph.D. thesis, Chalmers University of Technology, Gothenburg, Sweden, 1975.